



**INCON**

**INCON – Shear Studs (ISS)**



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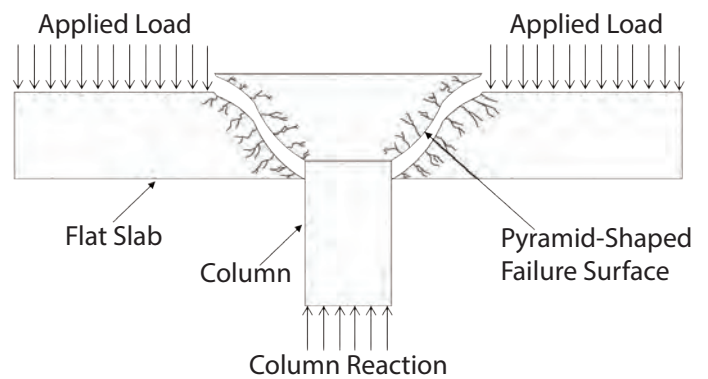
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Go Higher and Faster  
with - ISS

# Punching Shear Concept

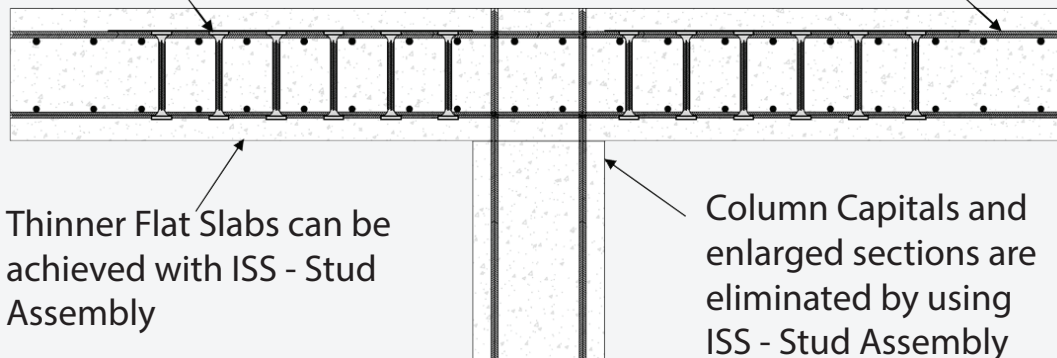
## ■ Punching Shear in Flat Slabs:

- A flat slab is a concrete slab of uniform thickness reinforced in two or more directions and supported directly by columns without beams or girders.
- Shear at column locations governs the thickness of a flat slab. Punching shear is the potentially high shearing stress developed by the reactive force of a column on a reinforced concrete slab.
- These slabs may fail locally under the concentrated load, i.e. a concrete pyramid together with the load or column is punched out from the slab.
- **ISS – Shear Studs** are used as shear reinforcement to significantly increase the shear capacity of slabs and eliminate premature failure.



ISS - Stud Assemblies distributed in the vicinity of the Column

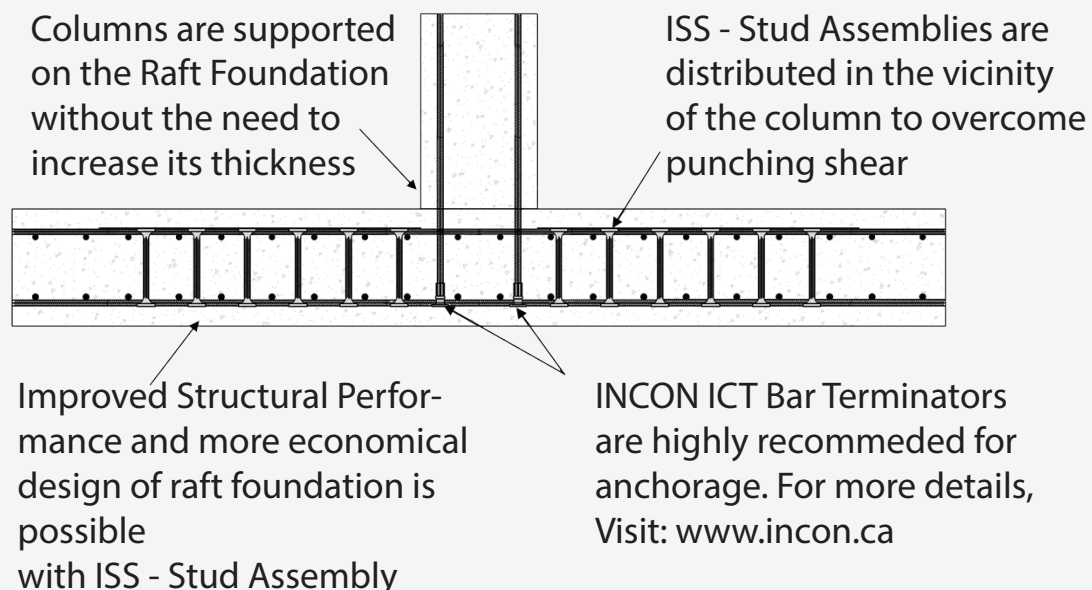
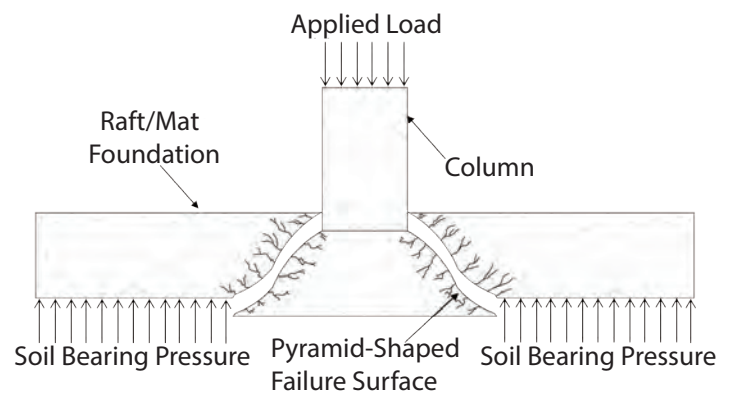
Flexural Reinforcement are utilized as supports to ISS - Stud Assembly



# Punching Shear Concept

## ■ Punching Shear in Raft/Mat Foundations:

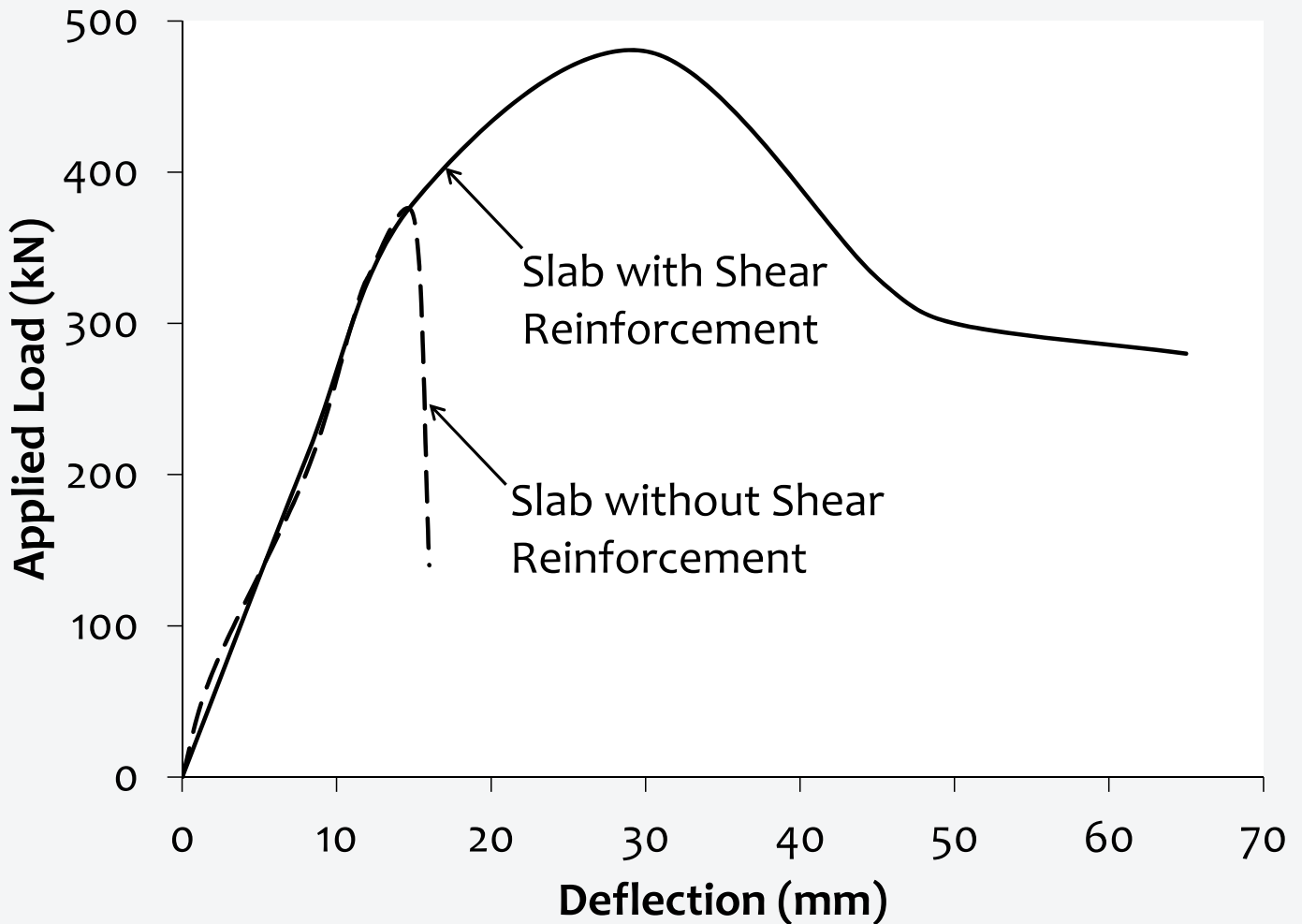
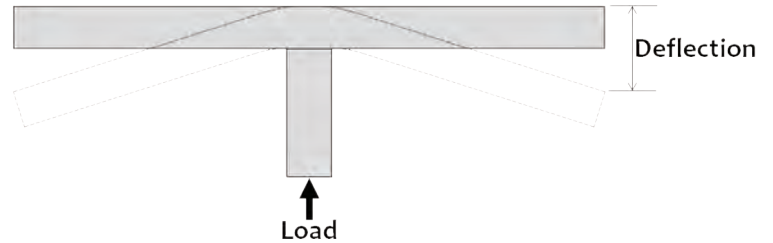
- A raft/mat foundation is a thick, heavily reinforced concrete slab that serves as a single monolithic footing for a number of columns or an entire building.
- Due to the existence of large soil pressure under the foundations, high shear stresses are developed in the vicinity of the columns. ISS - Shear Studs are used in order to overcome such stresses without increasing the thickness of the foundation.
- When shear studs are not used, the foundations may fail locally under the concentrated load in a manner that causes penetration of the columns through the foundations.
- **ISS – Shear Studs** are used as shear reinforcement to significantly increase the shear capacity of foundations in the vicinity of the columns and to eliminate premature failure at those sections.



# Shear Reinforcement

## ■ Importance of Shear Reinforcement:

- An experimental program\* performed on RC slab-column connections clearly shows the substantial influence of shear reinforcement on enhancing both punching shear resistance and ductility of the slabs.

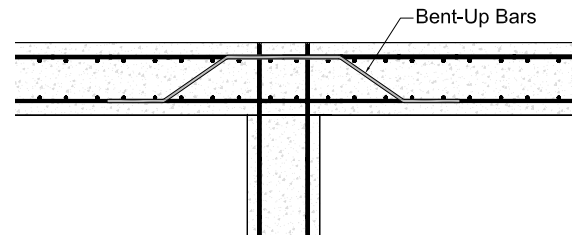


\* Dilger, WH and Ghali, A., 1981, Shear Reinforcement for Concrete Slabs, ASCE Journal of Structural Division, V. 107, pp. 2403-2420.

# Conventional Shear Reinforcement Methods

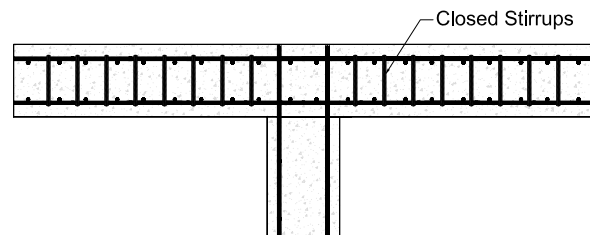
## Bent-Up Bars:

- Increase reinforcement congestion.
- Require special detailing.
- Increase cut and bend time.



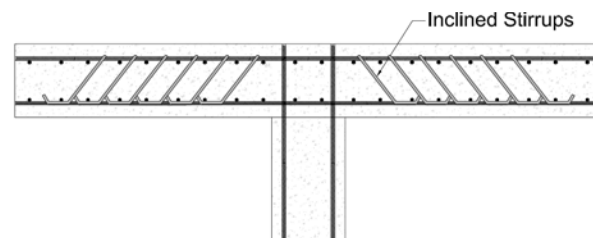
## Closed Stirrups:

- Increase reinforcement congestion.
- Require lengthy installation time.
- Increase cut and bend time.
- Labor Intensive



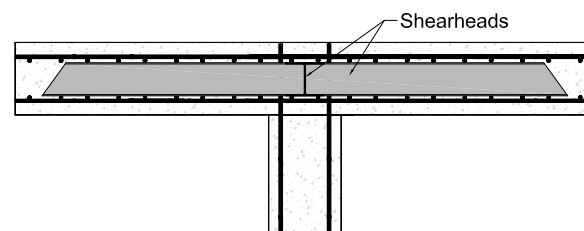
## Inclined Stirrups:

- Increase reinforcement congestion.
- Require special detailing.
- Increase cut and bend time.
- Increase installation time.
- Increase design duration due to the additional calculations involved.



## Shearheads:

- Act as barriers between bars.
- Require special detailing.
- Increase reinforcement congestion.
- Increase material cost.

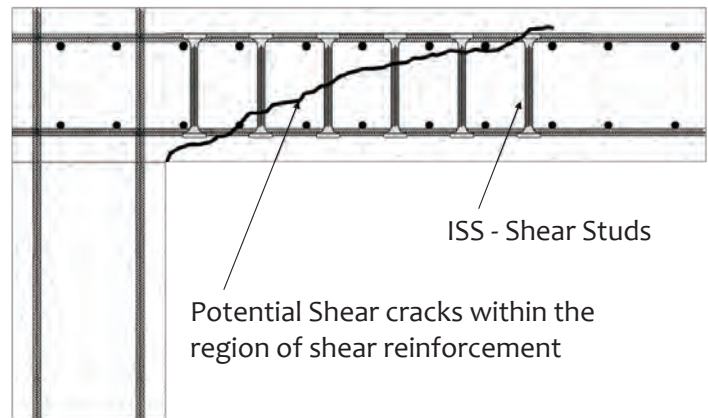


## Shear Failure Prevention with ISS – Shear Studs

### ■ First Mechanism:

**Potential shear cracks are within the region of shear reinforcement.**

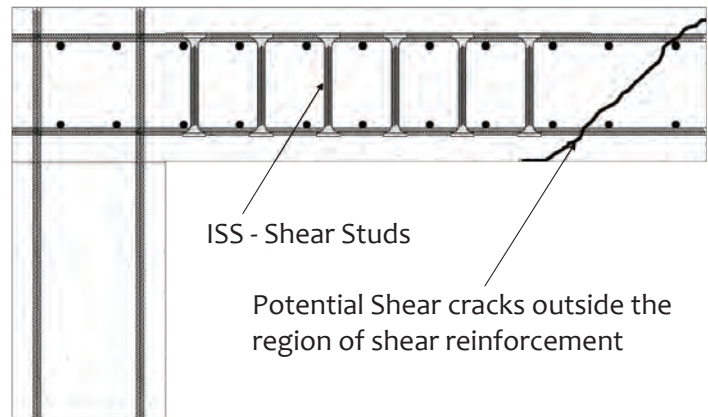
- ISS – Shear Studs are required to intercept shear cracks and prevent them from widening.



### ■ Second Mechanism:

**Potential shear cracks are outside the region of shear reinforcement.**

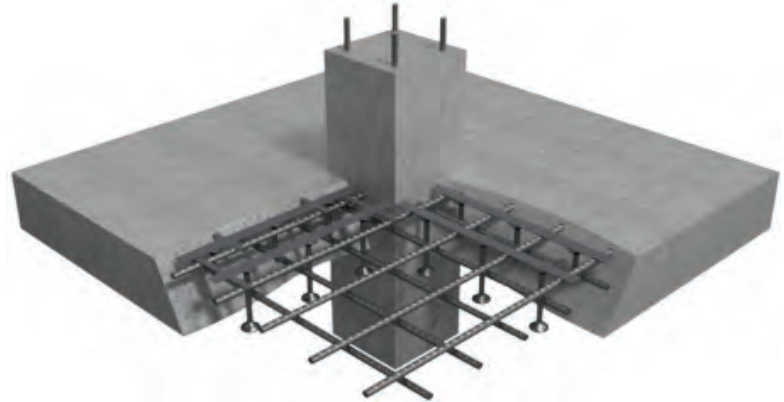
- Providing ISS – Shear Studs can hinder the formation of shear cracks within the shear reinforced zone and transfer them to the outer region where concrete possesses sufficient contact surface to resist shear stresses alone.





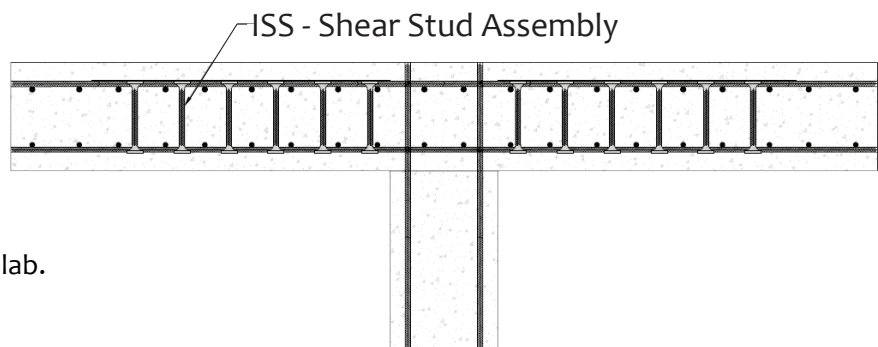
## ISS – Shear Stud Assemblies

- ISS – Shear Studs are manufactured under the strictest Canadian Quality Control guidelines and in accordance with ISO 9001:2008 Quality Assurance and Management System.
- ISS – Shear Studs provide an outstanding structural performance compared to conventional shear reinforcement due to the enhanced mechanical anchorage provided by their headed ends.
- This improved end anchorage controls the shear crack width and therefore enhances aggregate interlock between the two shear faces.
- When double headed studs are used in flat slabs with continuous top and bottom reinforcement over the column, this also improves the ductility and therefore the shear force at which the diagonal shear crack forms.



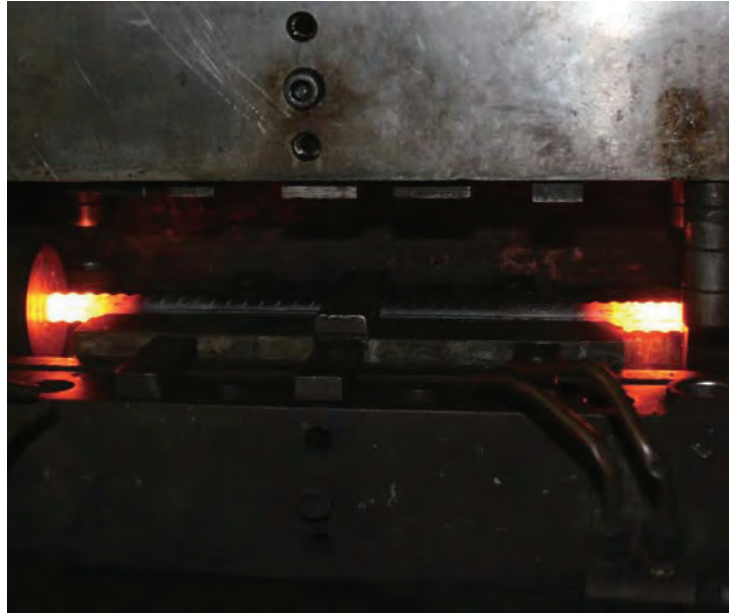
### Main Advantages:

- Reduce reinforcement congestion.
- Decrease Construction Time.
- Eliminate extra cut and bend works.
- Enhance structural performance.
- Allow greater design flexibility.
- Reduce slab thickness.
- Provide enhanced anchorage to the slab.
- Ensure factory quality control.
- Reduce material and labor cost.
- Easy to design using **ISS Software**.
- Easy to check and modify arrangement.



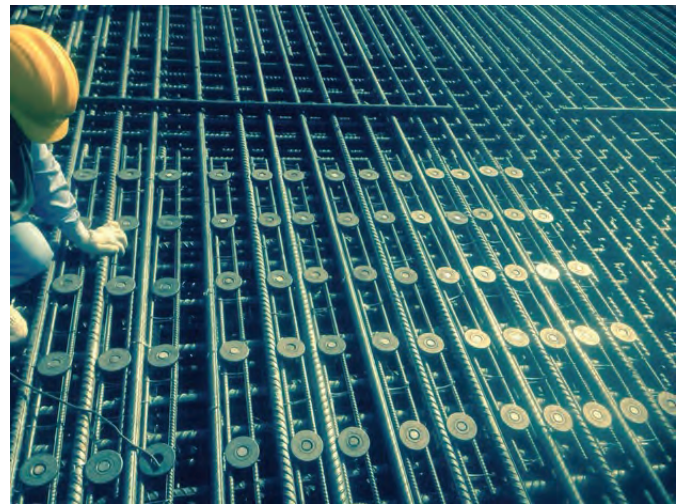
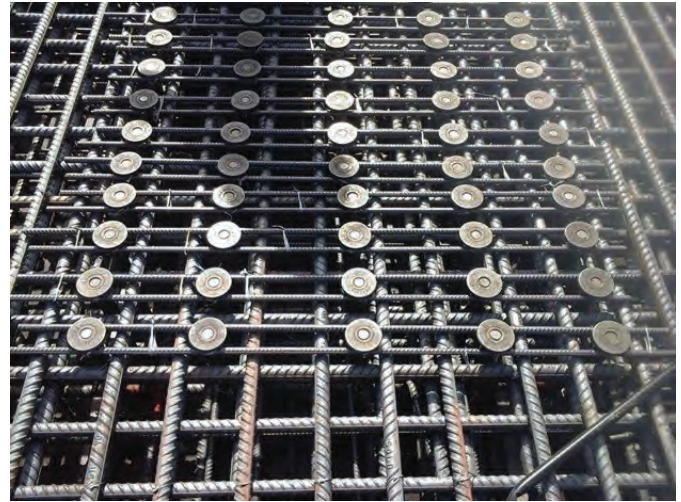
## Production of ISS - Shear Studs

- An ISS - Shear Stud consists of a deformed reinforcing bar which is hot forged from both sides.
- Studs are manufactured from steel conforming to ASTM A615 and ASTM A706, Grade 60.
- ISS Base Rail is manufactured in accordance with ASTM A1044 to be sufficiently stiff to hold the studs in the appropriate location, direction and spacing.
- The manufacturing process ensures outstanding structural performance of ISS – Shear Stud Assemblies.
- By implementing value engineering in our design and manufacturing process, we ensure that our clients receive high quality products with minimum cost.
- Finished ISS – Shear Studs are of uniform quality and condition, free of discontinuous laps, seams, fins, or other discontinuities.

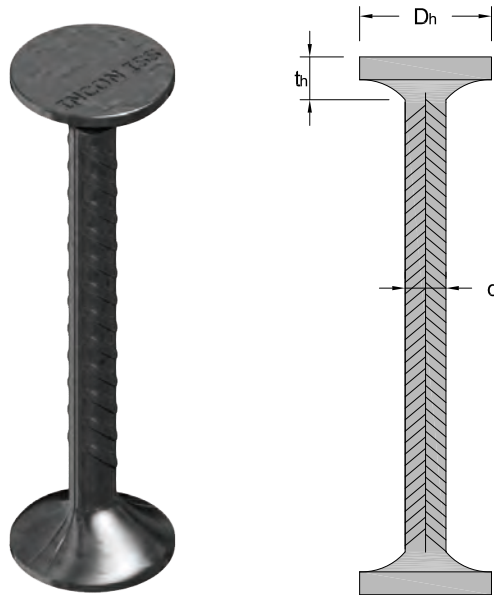


## Characteristics of ISS - Shear Studs

- ISS - Shear Studs are mechanically anchored at each end by a head bearing against the concrete in such a way that it is capable of developing the full yield strength of the bar.
- The dimensions of ISS – Shear Studs are chosen to satisfy the requirements of ASTM A1044, ACI 318-14 and CSA A23.3-14 by ensuring that the area of the head is at least ten times the cross-sectional area of the shank.
- The length of ISS – Shear Studs are selected to cover the maximum possible depth in the slab since potential shear cracks can develop at any location over the stud height.



## Dimensions of ISS – Shear Studs



Sizes and Dimensions of ISS - Shear Studs (SI Units)			
Shank Diameter	Head Diameter	Head Thickness	Head Cross-Sectional Area
d (mm)	D <sub>h</sub> (mm)	t <sub>h</sub> (mm)	A (mm <sup>2</sup> )
9.5	30.2	5.3	716
12.7	40.4	7.1	1282
15.9	50.3	8.9	1987
19.1	60.5	10.7	2875
25.4	80.5	13.2	5090

Sizes and Dimensions of ISS - Shear Studs (Imperial Units)			
Shank Diameter	Head Diameter	Head Thickness	Head Cross-Sectional Area
d (in)	D <sub>h</sub> (in)	t <sub>h</sub> (in)	A (in <sup>2</sup> )
0.375	1.19	0.21	1.1
0.500	1.59	0.28	2.0
0.625	1.98	0.35	3.1
0.750	2.38	0.42	4.4
1.000	3.17	0.52	7.9

## Acceptance Criteria of ISS - Shear Studs

- At periodic intervals, tensile tests of the ISS Shear Studs are performed as specified by INCONS quality assurance program in accordance with ASTM A370.
- The tensile tests are conducted in our factory and by a third party laboratory in order to ensure that our products always exceed the strictest international standards.



### In all of the tested specimens:

- Failure occurs in ISS – Shear Stud material a minimum of one-half shank diameter from the head-to-shank connection as required by ASTM A1044.
- The tensile force at failure always exceeds the specified yield strength of ISS - Shear Stud material.
- No partial or total fracture of ISS – Shear Stud head is observed.



# Design of ISS – Shear Studs

The design methodology for punching shear using ISS – Shear Studs is performed in accordance with ACI 318-14 and CSA A23.3-14.

The following illustrates the design procedure for a typical flat slab:

## Formulas and Design Procedure:

### 1- Inputs are:

- Concrete Compressive Strength ( $f'_c$ )
- Studs Yield Strength ( $f_y$ )
- Concrete Type Coefficient ( $\lambda$ )
- Slab Thickness ( $h$ )
- Concrete Cover ( $cc$ )
- Applied Shear Force ( $V_u$ )
- Applied Moment ( $M_u$ )

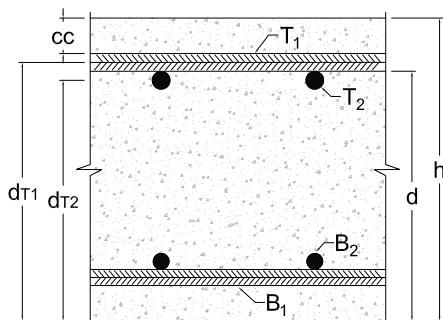
## Case Study:

- $f'_c = 4000$  psi
- $f_y = 51$  ksi
- $\lambda = 1$  (Normal Weight Concrete)
- $h = 7$  in.
- $cc = 0.75$  in.
- $V_u = 110$  Kips
- $M_u = 600$  Kips.in

### 2- Determine the effective depth of the slab.

$$d = h - cc - \frac{D_{T1} + D_{T2}}{2}$$

$$d = 7 - 0.75 - \frac{0.625 + 0.625}{2} = 5.625 \text{ in}$$



## Design of ISS – Shear Studs

### 3- Determine the location and geometry of the critical sections.

- The perimeter ( $b_o$ ) of the critical section is chosen such that it minimizes the shear surface area at a distance of not less than  $(d/2)$  to edges or corners of columns.
- If an opening is located in the vicinity of the column, then a portion the critical section is considered ineffective as detailed in ACI 318-14.
- The critical shear perimeters for interior, edge and corner columns are shown in Figures 1, 2 and 3, respectively.

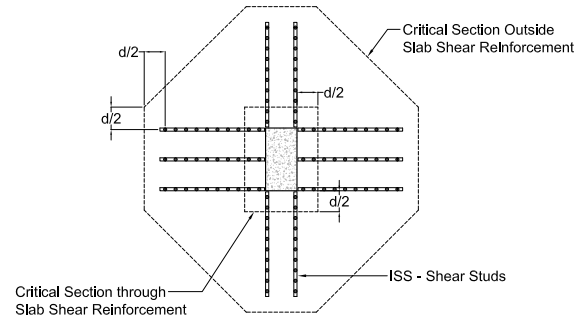


Fig. 1: Critical Perimeters for an Interior Column.

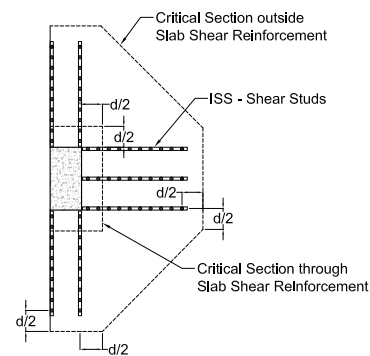


Fig. 2: Critical Perimeters for an Edge Column.

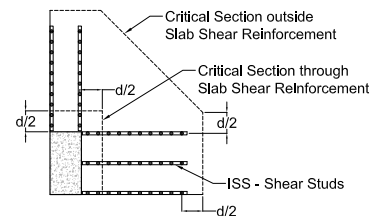


Fig. 3: Critical Perimeters for a Corner Column.

## Design of ISS – Shear Studs

- 4- Calculate the maximum factored shear stress ( $v_u$ ) at the critical section.

$$v_u = 295.0 \text{ psi}$$

- 5- Calculate the nominal shear strength ( $v_n$ ) at the critical section.

$$\beta = \frac{\text{Long Column Side}}{\text{Short Column Side}} = \frac{20}{12} = 1.67$$

$$\alpha_s = 40 \text{ for interior columns}$$

$$v_n = \min \left\{ \begin{array}{l} \left(2 + \frac{4}{\beta}\right) \lambda \sqrt{f'_c} \\ \left(\frac{\alpha_s d}{b_o} + 2\right) \lambda \sqrt{f'_c} \\ 4 \lambda \sqrt{f'_c} \end{array} \right.$$

$$v_n = \min \left\{ \begin{array}{l} \left(2 + \frac{4}{1.67}\right) \lambda \sqrt{f'_c} = 4.4 \lambda \sqrt{f'_c} \\ \left(\frac{40 \times 5.625}{86.5} + 2\right) \lambda \sqrt{f'_c} = 4.6 \lambda \sqrt{f'_c} \\ 4 \lambda \sqrt{f'_c} \end{array} \right.$$

Where  $\alpha_s$  and  $\beta$  are factors given in ACI 318-14 in terms of column location and geometrical properties, respectively.

$$\therefore v_n = 2.6 \lambda \sqrt{f'_c} = 4 \times 1 \times \sqrt{4000} = 253 \text{ psi}$$

- 6- Compare the maximum factored shear stress ( $v_u$ ) to the nominal shear strength ( $v_n$ ).

$$v_u = 295.0 \text{ psi}$$

$$\phi v_n = 0.75 \times 253 = 189.7 \text{ psi}$$

- If ( $v_u \leq \phi v_n$ ), then no shear reinforcement or further check is required.
- If ( $v_u \leq \phi \times 8 \sqrt{f'_c}$ ), then shear studs can be used to resist punching shear (Proceed to next step).
- If ( $v_u > \phi \times 8 \sqrt{f'_c}$ ), then slab thickness is not sufficient and should be increased.

$$\begin{aligned} \therefore (v_u = 295.0 \text{ psi}) &\leq (\phi v_n = 189.7 \text{ psi}) \\ \therefore \text{Concrete contribution is not sufficient} \end{aligned}$$

$$\phi \times 8 \sqrt{f'_c} = 0.75 \times 8 \times \sqrt{4000} = 379.4 \text{ psi}$$

$$\begin{aligned} \therefore (v_u = 295.0 \text{ psi}) &\leq (\phi \times 8 \sqrt{f'_c} = 379.4 \text{ psi}) \\ \therefore \text{Shear studs can be used} \end{aligned}$$



## Design of ISS – Shear Studs

- 7- Calculate the concrete contribution to shear strength ( $v_c$ ) at the critical section.

$$v_c = 3\lambda\sqrt{f'_c}$$

$$v_c = 3 \times 1 \times \sqrt{4000} = 189.7 \text{ psi}$$

- 8- Calculate the shear stress ( $v_s$ ) to be resisted by the shear studs.

$$v_s = \frac{v_u}{\phi} - v_c$$

$$v_s = \frac{295.0}{0.75} - 189.7 = 203.6 \text{ psi}$$

- 9- Select the spacing ( $s_o$ ) between first peripheral line of shear studs and column face.

$$0.35 d \leq (s_o)_{max} \leq 0.50 d$$

$$0.35 d = 0.35 \times 5.625 = 1.97 \text{ in}$$

$$0.50 d = 0.50 \times 5.625 = 2.81 \text{ in}$$

$\therefore$  Choose  $s_o = 2.25 \text{ in}$

- 10- Select the spacing ( $s$ ) between peripheral lines of shear studs.

- $s \leq 0.75 d$ , when  $v_u \leq \phi \times 6\sqrt{f'_c}$
- $s \leq 0.50 d$ , when  $v_u > \phi \times 6\sqrt{f'_c}$

$$v_u = 295.0 \text{ psi}$$

$$\phi \times 6\sqrt{f'_c} = 0.75 \times 6 \times \sqrt{4000} = 284.6 \text{ psi}$$

$$\therefore (v_u = 295.0 \text{ psi}) > (\phi \times 6\sqrt{f'_c} = 284.6 \text{ psi})$$

$$\therefore s \leq (0.50 d = 0.50 \times 5.625 = 2.81 \text{ in})$$

$\therefore$  Choose  $s = 2.75 \text{ in}$

- 11- Calculate the required shear stud area ( $A_v$ ) for one peripheral line.

$$A_v = \frac{v_s b_o s}{f_y}$$

$$A_v = \frac{203.6 \times 86.5 \times 2.75}{51000} = 0.95 \text{ in}^2$$

## Design of ISS – Shear Studs

- 12- Determine the minimum number of shear studs per peripheral line.

Select ISS studs of 0.11 in<sup>2</sup> cross-sectional area

$$N_{Studs} = \frac{A_v}{A_{stud}} = \frac{0.95}{0.11} = 8.64 \cong 9 \text{ studs}$$

- 13- Through trial and error, find the section where  $v_u \leq \phi \times 2\sqrt{f'_c}$  which is located at a distance of ( $\alpha d$ ) from the face of the column.

For First Trial:

Assume critical section is located at 4.5d from column face.

$$\alpha = 4.5$$

$$\alpha d = 4.5 \times 5.625 = 25.3 \text{ in}$$

$$\begin{aligned} l_{x2} &= c_x + 2s_o + 2(N_{Studs} - 1)s + d \\ l_{x2} &= 12 + 2 \times 2.25 + 2(9 - 1) \times 2.75 + 5.625 \\ \therefore l_{x2} &= 66.1 \text{ in} \end{aligned}$$

$$\begin{aligned} l_{y2} &= c_y + 2s_o + 2(N_{Studs} - 1)s + d \\ l_{y2} &= 20 + 2 \times 2.25 + 2(9 - 1) \times 2.75 + 5.625 \\ \therefore l_{y2} &= 74.1 \text{ in} \end{aligned}$$

$$\begin{aligned} \gamma_{vy} &= 1 - \frac{1}{1 + \frac{2}{3}\sqrt{l_{x2}/l_{y2}}} \\ &= 1 - \frac{1}{1 + \frac{2}{3}\sqrt{66.1/74.1}} \end{aligned}$$

$$\therefore \gamma_{vy} = 0.386$$

$$\begin{aligned} b_o &= 2(c_x + d) + 2(c_y + d) \\ &\quad + 4[s_o + (N_{Studs} - 1)s] \\ b_o &= 2(12 + 5.625) + 2(20 + 5.625) \\ &\quad + 4[2.25 + (9 - 1)2.75] \\ \therefore b_o &= 183.5 \text{ in} \end{aligned}$$

$$A_c = b_o d = 183.5 \times 5.625 = 1032.1 \text{ in}^2$$

$$J_y = 547.3 \times 10^3 \text{ in}^4$$

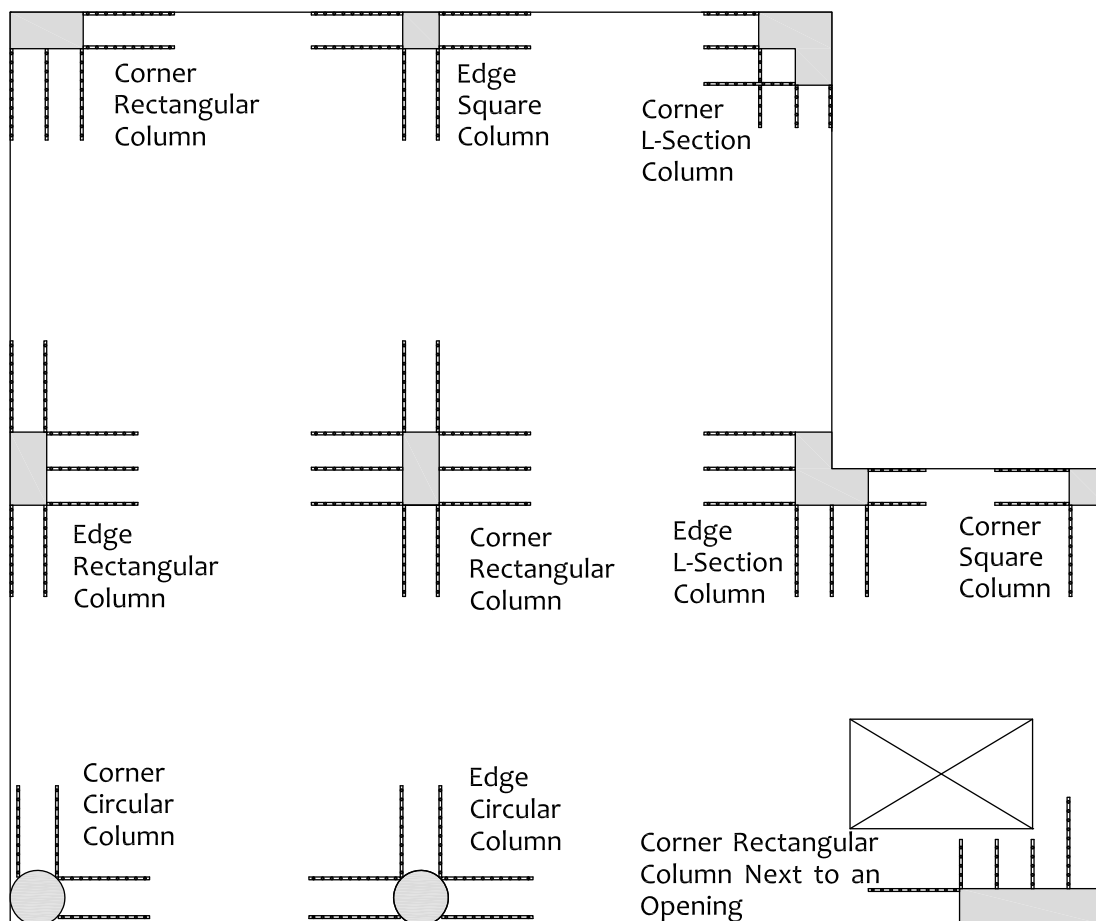
## Design of ISS – Shear Studs

- 14- Select the distance between the column face and the outermost peripheral line of shear studs to be greater than or equal to  $(\alpha d - \frac{d}{2})$ .

- 15- The position of the critical section can be determined by selection of the number ( $n$ ) of shear studs per line running in ( $x$ ) or ( $y$ ) direction.

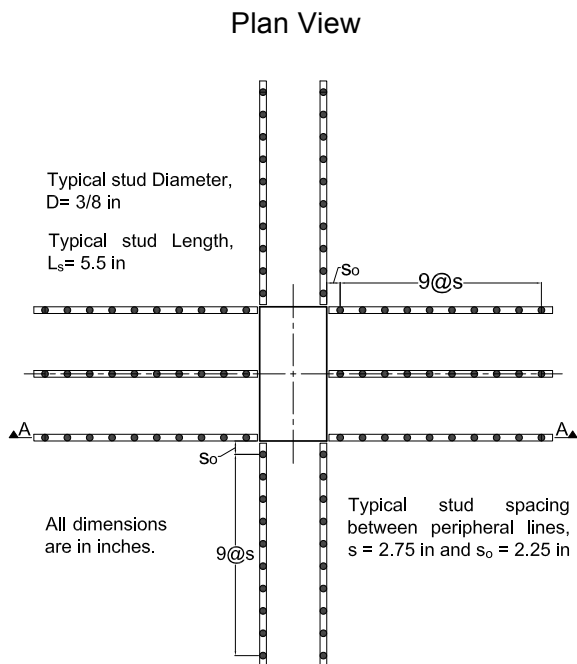
$$L_c = s_o + (n - 1)s + \frac{d}{2}$$

- 16- Arrange the studs to satisfy ACI 318-14 requirements.

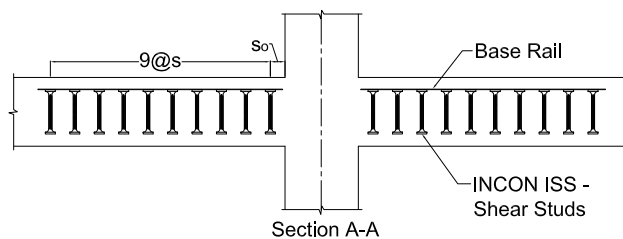


# Typical Arrangement of ISS - Shear Studs

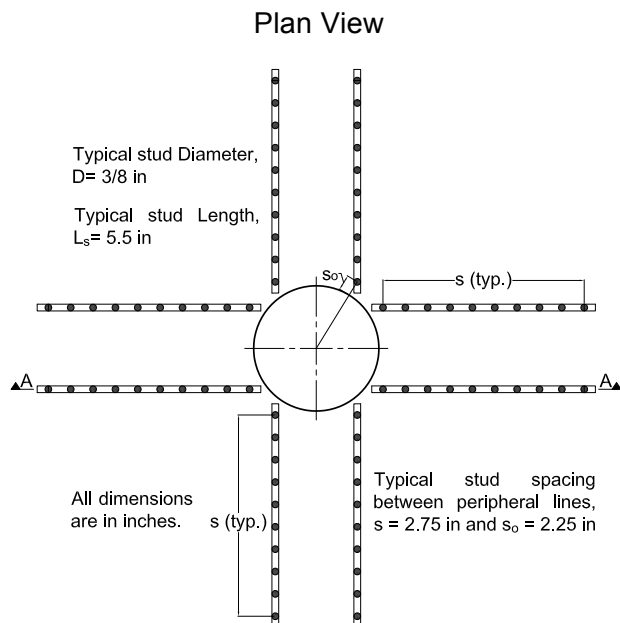
- Interior Rectangular Columns:



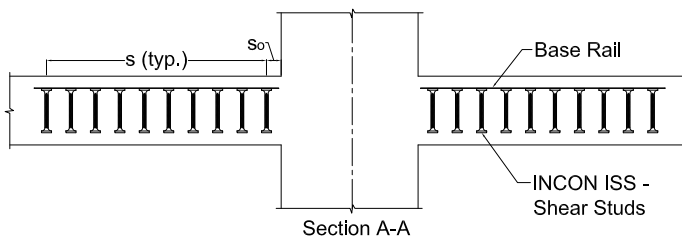
**Elevation View**



- Interior Circular Columns:

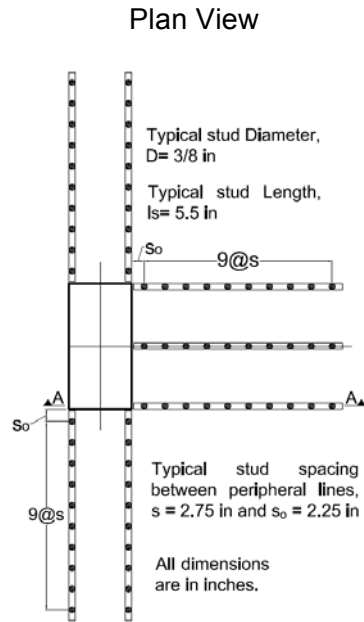


**Elevation View**

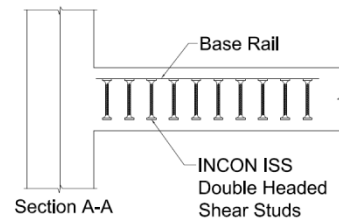


# Typical Arrangement of ISS - Shear Studs

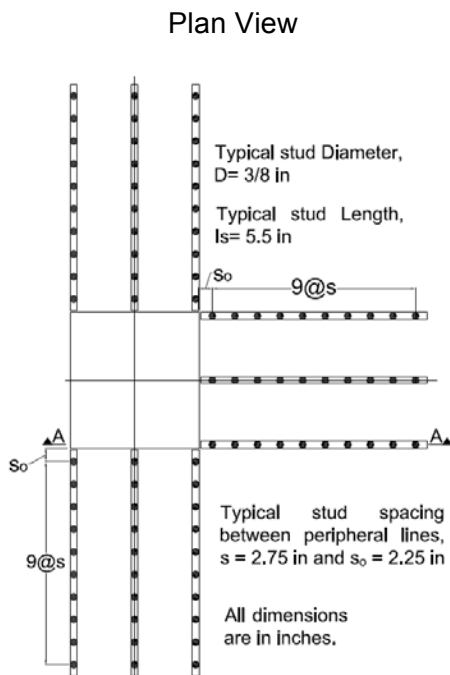
- Edge Rectangular Columns:



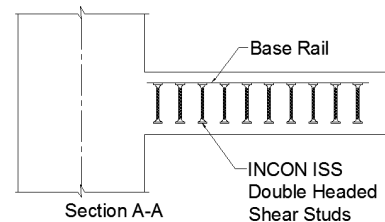
Elevation View



- Edge Square Columns:



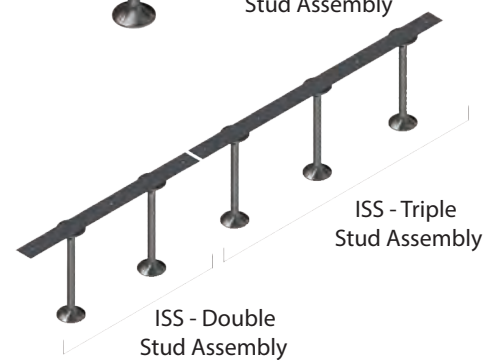
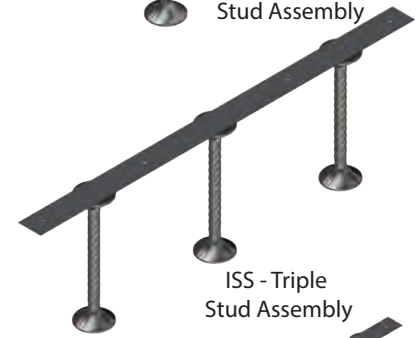
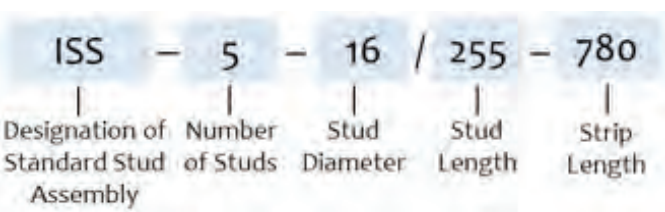
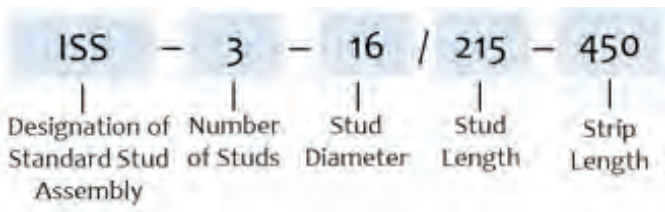
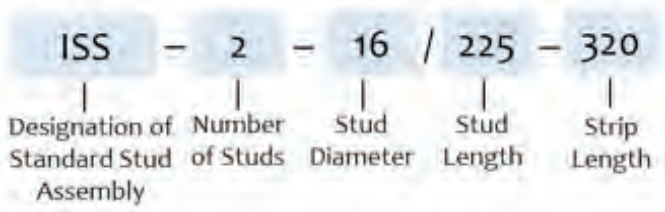
Elevation View



# Designation of ISS – Shear Stud Assemblies

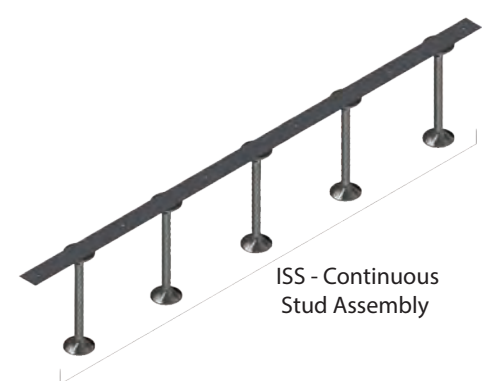
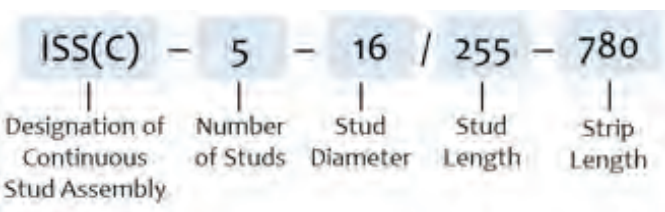
## OPTION 1: Standard ISS-Stud Assemblies

- Available as 2-stud or 3-stud assemblies.
- Several ISS 2-stud rails or ISS 3-stud rails can be combined to form a complete arm.



## OPTION 2: Continuous ISS-Stud Assemblies

- Available on request up to a maximum of 10 shear studs per rail.



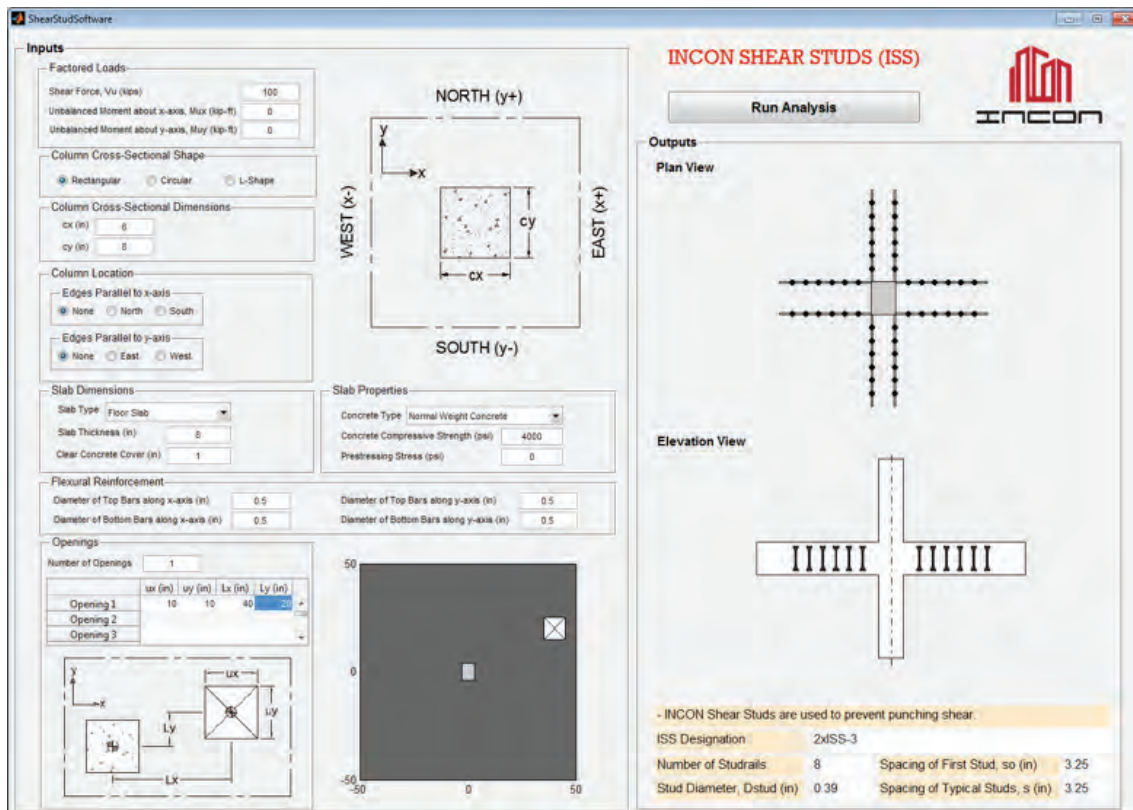
## Designation of ISS – Shear Studs

After designing the slab for punching shear, the following table can be used to determine different possible arrangements of ISS – Shear Studs per arm.

Number of ISS - Shear Studs per Arm	Alternative 1	Alternative 2	Alternative 3	Alternative 4
2	ISS-2	N.A.	N.A.	N.A.
3	ISS-3	N.A.	N.A.	N.A.
4	2 x ISS-2	ISS(C)-4	N.A.	N.A.
5	ISS-2 + ISS-3	ISS(C)-5	N.A.	N.A.
6	2 x ISS-3	3 x ISS-2	ISS(C)-6	N.A.
7	2 x ISS-2 + ISS-3	ISS(C)-7	ISS(C)-4 + ISS-3	N.A.
8	2 x ISS-3 + ISS-2	4 x ISS-2	ISS(C)-8	ISS(C)-5 + ISS-3
9	3 x ISS-3	3 x ISS-2 + ISS-3	ISS(C)-9	ISS(C)-5 + 2 x ISS-2
10	2 x ISS-2 + 2 x ISS-3	2 x ISS(C)-5	ISS(C)-10	2 x ISS(C)-4 + ISS-2
11	3 x ISS-3 + ISS-2	ISS(C)-9 + ISS-2	ISS(C)-8 + ISS-3	ISS(C)-7 + 2 x ISS-2
12	4 x ISS-3	6 x ISS-2	2 x ISS(C)-6	ISS(C)-10 + ISS-2
13	3 x ISS-3 + 2 x ISS-2	5 x ISS-2 + ISS-3	2 x ISS(C)-5 + ISS-3	ISS(C)-10 + ISS-3
14	4 x ISS-3 + ISS-2	7 x ISS-2	2 x ISS(C)-7	ISS(C)-10 + 2 x ISS-2
15	5 x ISS-3	6 x ISS-2 + ISS-3	3 x ISS(C)-5	ISS(C)-10 + ISS-3 + ISS-2
16	4 x ISS-3 + 2 x ISS-2	2 x ISS(C)-8	4 x ISS-4	ISS(C)-10 + 2 x ISS-3
17	5 x ISS-3 + ISS-2	2 x ISS(C)-7 + ISS-3	3 x ISS(C)-5 + ISS-2	ISS(C)-10 + ISS(C)-7
18	6 x ISS-3	3 x ISS(C)-5 + ISS-3	3 x ISS(C)-6	ISS(C)-10 + ISS(C)-8
19	5 x ISS-3 + 2 x ISS-2	3 x ISS(C)-5 + 2 x ISS-2	2 x ISS(C)-8 + ISS-3	ISS(C)-10 + ISS(C)-9
20	6 x ISS-3 + ISS-2	4 x ISS(C)-5	2 x ISS(C)-10	2 x ISS(C)-7 + 2 x ISS-3

## How to Order?

- To ensure the most efficient and smooth construction flow, ASTM 1044-16 requires that the purchaser provide a detailed description of the required Shear Stud Assembly when placing the order.
- ISS Software simplifies this process by allowing the engineers to easily prepare schedules that summarize the quantity and geometrical properties of each ISS - Shear Stud Assembly at each column.
- As an alternative, engineers can specify required ISS – Stud Assembly using the tables on pages 25 through 30.

**Inputs**

**Factored Loads**

Shear Force,  $V_u$  (kips) 100  
 Unbalanced Moment about x-axis,  $M_{ux}$  (kip-ft) 0  
 Unbalanced Moment about y-axis,  $M_{uy}$  (kip-ft) 0

**Column Cross-Sectional Shape**

Rectangular  Circular  L-Shape

**Column Cross-Sectional Dimensions**

cx (in) 8  
 cy (in) 8

**Column Location**

Edges Parallel to x-axis  
 None  North  South

Edges Parallel to y-axis  
 None  East  West

**Slab Dimensions**

Slab Type Floor Slab  
 Slab Thickness (in) 8  
 Clear Concrete Cover (in) 1

**Slab Properties**

Concrete Type Normal Weight Concrete  
 Concrete Compressive Strength (psi) 4000  
 Prestressing Stress (psi) 0

**Flexural Reinforcement**

Diameter of Top Bars along x-axis (in) 0.5  
 Diameter of Bottom Bars along x-axis (in) 0.5  
 Diameter of Top Bars along y-axis (in) 0.5  
 Diameter of Bottom Bars along y-axis (in) 0.5

**Openings**

Number of Openings 1

Opening	ux (in)	uy (in)	Lx (in)	Ly (in)
Opening 1	10	10	40	
Opening 2				
Opening 3				

**INCON SHEAR STUDS (ISS)**

**Run Analysis**

**Outputs**

**Plan View**

**Elevation View**

- INCON Shear Studs are used to prevent punching shear.

ISS Designation	2xISS-3		
Number of Studrails	8	Spacing of First Stud, $s_o$ (in)	3.25
Stud Diameter, $D_{stud}$ (in)	0.39	Spacing of Typical Studs, $s$ (in)	3.25



## INCON Services

### Design Services:

- Our design service enables us to accurately estimate project costs and speedily provide quotations.
- In addition to this service, INCON also provides design software for clients who prefer to produce their own calculations and layouts.



### Delivery to Site:

- After preparing ISS - Shear Stud Assemblies, they will be packaged in containers clearly marked with the assembly part number and description to facilitate the installation process on site.
- The description provided on each package indicates the base rail width, thickness, length, and the number of headed studs with their diameter for each rail.

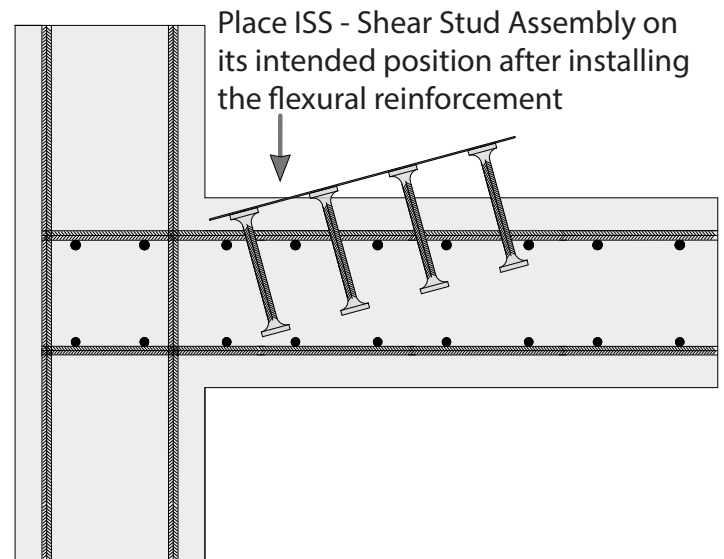


## Installation of ISS – Shear Studs

When installing ISS – Shear Studs on site, the rails can be fitted as follows:

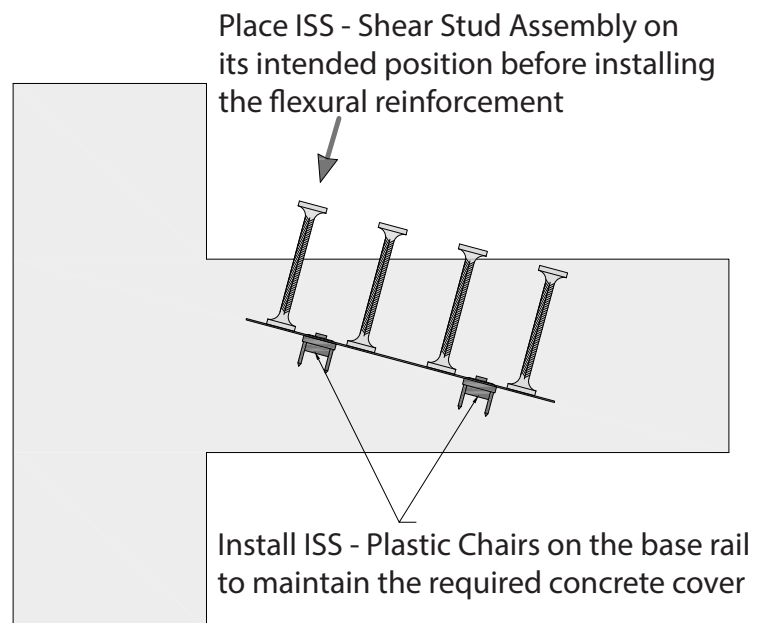
### • Top down:

- 1- Install the upper and bottom layers of flexural reinforcement.
- 2- Place ISS – Shear Studs through the reinforcing bars according to the structural drawings.
- 3- Ensure that the base rail rests on the uppermost reinforcing bars either directly or indirectly using special ISS supporting plates. These plates can be provided separately on request.
- 4- Connect the base rail to the flexural reinforcing bars using tie wires.



### • Bottom up:

- 1- Attach ISS – Plastic Chairs on the base rail depending on the required clear concrete cover.
- 2- Place ISS – Stud Assembly on intended location to form shear arms in the slab around the columns.
- 3- Fix the Shear Studs to the formwork using an approved method by the engineering team.
- 4- Install the flexural reinforcement according to the structural drawings.



# INCON Organizational Memberships

INCON is proudly an active member in the most prestigious organizations and committees throughout Canada and the United States.



## Design Aids

The overall height and maximum spacing between ISS – Shear Studs can be determined using the tables detailed in pages 25 through 30 in terms of slab thickness, flexural reinforcement and clear concrete cover.

Page 25	Actual Overall Height of ISS – Shear Studs according to Canadian Standards.
Page 26	Actual Overall Height of ISS – Shear Studs according to American Standards.
Page 27	Actual Overall Height of ISS – Shear Studs according to European and the Middle Eastern Standards.
Page 28	Maximum Spacing of ISS – Shear Studs according to Canadian Standards.
Page 29	Maximum Spacing of ISS – Shear Studs according to American Standards.
Page 30	Maximum Spacing of ISS – Shear Studs according to European and the Middle Eastern Standards.

The Following Table Can be used to Determine the Actual Overall Height of ISS-Shear Studs:



CANADA

Flexural Bar Designation	10M			15M			20M			25M			30M			35M			45M		
	20	30	75	20	30	75	20	30	75	20	30	75	20	30	75	20	30	75	20	30	75
Clear Concrete Cover (mm)	74	54	N.A.	72	52	N.A.	70	50	N.A.	67	47	N.A.	65	45	N.A.	62	42	N.A.	58	38	N.A.
120	104	84	N.A.	102	82	N.A.	100	80	N.A.	97	77	N.A.	95	75	N.A.	92	72	N.A.	88	68	N.A.
150	134	114	N.A.	132	112	N.A.	130	110	N.A.	127	107	N.A.	125	105	N.A.	122	102	N.A.	118	98	N.A.
180	154	134	44	152	132	42	150	130	40	147	127	37	145	125	35	142	122	32	138	118	28
200	179	159	69	177	157	67	175	155	65	172	152	62	170	150	60	167	147	57	163	143	53
225	204	184	94	202	182	92	200	180	90	197	177	87	195	175	85	192	172	82	188	168	78
250	229	209	119	227	207	117	225	205	115	222	202	112	220	200	110	217	197	107	213	193	103
275	254	234	144	252	232	142	250	230	140	247	227	137	245	225	135	242	222	132	238	218	128
300	279	259	169	277	257	167	275	255	165	272	252	162	270	250	160	267	247	157	263	243	153
325	304	284	194	302	282	192	300	280	190	297	277	187	295	275	185	292	272	182	288	268	178
350	329	309	219	327	307	217	325	305	215	322	302	212	320	300	210	317	297	207	313	293	203
375	354	334	244	352	332	242	350	330	240	347	327	237	345	325	235	342	322	232	338	318	228
400	379	359	269	377	357	267	375	355	265	372	352	262	370	350	260	367	347	257	363	343	253
425	404	384	294	402	382	292	400	380	290	397	377	287	395	375	285	392	372	282	388	368	278
450	429	409	319	427	407	317	425	405	315	422	402	312	420	400	310	417	397	307	413	393	303
475	454	434	344	452	432	342	450	430	340	447	427	337	445	425	335	442	422	332	438	418	328
500	479	459	369	477	457	367	475	455	365	472	452	362	470	450	360	467	447	357	463	443	353
525	504	484	394	502	482	392	500	480	390	497	477	387	495	475	385	492	472	382	488	468	378
550	529	509	419	527	507	417	525	505	415	522	502	412	520	500	410	517	497	407	513	493	403
575	554	534	444	552	532	442	550	530	440	547	527	437	545	525	435	542	522	432	538	518	428
600	579	559	469	577	557	467	575	555	465	572	552	462	570	550	460	567	547	457	563	543	453
625	604	584	494	602	582	492	600	580	490	597	577	487	595	575	485	592	572	482	588	568	478
650	629	609	519	627	607	517	625	605	515	622	602	512	620	600	510	617	597	507	613	593	503
675	654	634	544	652	632	542	650	630	540	647	627	537	645	625	535	642	622	532	638	618	528
700	704	684	594	702	682	592	700	680	590	697	677	587	695	675	585	692	672	582	688	668	578
750	754	734	644	752	732	642	750	730	640	747	727	637	745	725	635	742	722	632	738	718	628
800	804	784	694	802	782	692	800	780	690	797	777	687	795	775	685	792	772	682	788	768	678
850	854	834	744	852	832	742	850	830	740	847	827	737	845	825	735	842	822	732	838	818	728
900	904	884	794	902	882	792	900	880	790	897	877	787	895	875	785	892	872	782	888	868	778
950	954	934	844	952	932	842	950	930	840	947	927	837	945	925	835	942	922	832	938	918	828
1000	1004	984	894	1002	982	892	1000	980	890	997	977	887	995	975	885	992	972	882	988	968	878
1050	1054	1034	944	1052	1032	942	1050	1030	940	1047	1027	937	1045	1025	935	1042	1022	932	1038	1018	928
1100	1104	1084	994	1102	1082	992	1100	1080	990	1097	1077	987	1095	1075	985	1092	1072	982	1088	1068	978
1150	1154	1134	1044	1152	1132	1042	1150	1130	1040	1147	1127	1037	1145	1125	1035	1142	1122	1032	1138	1118	1028
1200	1204	1184	1094	1202	1182	1092	1200	1180	1090	1187	1167	1077	1185	1165	1075	1182	1162	1072	1178	1158	1068

Slab Thickness (mm)





The Following Table Can be used to Determine the Maximum Spacing between ISS-Shear Studs:



CANADA

Flexural Bar Designation	10M			15M			20M			25M			30M			35M			45M		
	20	30	75	20	30	75	20	30	75	20	30	75	20	30	75	20	30	75	20	30	75
Clear Concrete Cover (mm)	44	39	N.A.	42	37	N.A.	40	35	N.A.	37	32	N.A.	35	30	N.A.	32	27	N.A.	28	23	N.A.
	59	54	N.A.	57	52	N.A.	55	50	N.A.	52	47	N.A.	50	45	N.A.	47	42	N.A.	43	38	N.A.
	74	69	N.A.	72	67	N.A.	70	65	N.A.	67	62	N.A.	65	60	N.A.	62	57	N.A.	58	53	N.A.
	84	79	57	82	77	55	80	75	53	77	72	50	75	70	48	72	67	45	68	63	41
	97	92	69	95	90	67	93	88	65	90	85	62	88	83	60	85	80	57	81	76	53
	109	104	82	107	102	80	105	100	78	102	97	75	100	95	73	97	92	70	93	88	66
	122	117	94	120	115	92	118	113	90	115	110	87	113	108	85	110	105	82	106	101	78
	134	129	107	132	127	105	130	125	103	127	122	100	125	120	98	122	117	95	118	113	91
	147	142	119	145	140	117	143	138	115	140	135	112	138	133	110	135	130	107	131	126	103
	159	154	132	157	152	130	155	150	128	152	147	125	150	145	123	147	142	120	143	138	116
	172	167	144	170	165	142	168	163	140	165	160	137	163	158	135	160	155	132	156	151	128
	184	179	157	182	177	155	180	175	153	177	172	150	175	170	148	172	167	145	168	163	141
	197	192	169	195	190	167	193	188	165	190	185	162	188	183	160	185	180	157	181	176	153
	209	204	182	207	202	180	205	200	178	202	197	175	200	195	173	197	192	170	193	188	166
	222	217	194	220	215	192	218	213	190	215	210	187	213	208	185	210	205	182	206	201	178
	234	229	207	232	227	205	230	225	203	227	222	200	225	220	198	222	217	195	218	213	191
	247	242	219	245	240	217	243	238	215	240	235	212	238	233	210	235	230	207	231	226	203
	259	254	232	257	252	230	255	250	228	252	247	225	250	245	223	247	242	220	243	238	216
	272	267	244	270	265	242	268	263	240	265	260	237	263	258	235	260	255	232	256	251	228
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	309	304	282	307	302	280	305	300	278	302	297	275	300	295	273	297	292	270	293	288	266
	322	317	294	320	315	292	318	313	290	315	310	287	313	308	285	310	305	282	306	301	278
	334	329	307	332	327	305	330	325	303	327	322	300	325	320	298	322	317	295	318	313	291
	359	354	332	357	352	330	355	350	328	352	347	325	350	345	323	347	342	320	343	338	316
	384	379	357	382	377	355	380	375	353	377	372	350	375	370	348	372	367	345	368	363	341
	409	404	382	407	402	380	405	400	378	402	397	375	400	395	373	397	392	370	393	388	366
	434	429	407	432	427	405	430	425	403	427	422	400	425	420	398	422	417	395	418	413	391
	459	454	432	457	452	430	455	450	428	452	447	425	450	445	423	447	442	420	443	438	416
	484	479	457	482	477	455	480	475	453	477	472	450	475	470	448	472	467	445	468	463	441
	509	504	482	507	502	480	505	500	478	502	497	475	500	495	473	497	492	470	493	488	466
	534	529	507	532	527	505	530	525	503	527	522	500	525	520	498	522	517	495	518	513	491
	559	554	532	557	552	530	555	550	528	552	547	525	550	545	523	547	542	520	543	538	516
	584	579	557	582	577	555	580	575	553	577	572	550	575	570	548	572	567	545	568	563	541

Slab Thickness (mm)









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